

TECHNICAL REFERENCE

MOSU Dam Rehabilitation Project

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Governorate of Nineveh, Iraq

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Drilling & Grouting Civil & Hydromechanical Works

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MINISTRY OF WATER RESOURCES OF IRAQ
U.S. Army Corps of Engineers
Trevi S.p.A.
02/03/2016 - 12/08/2019

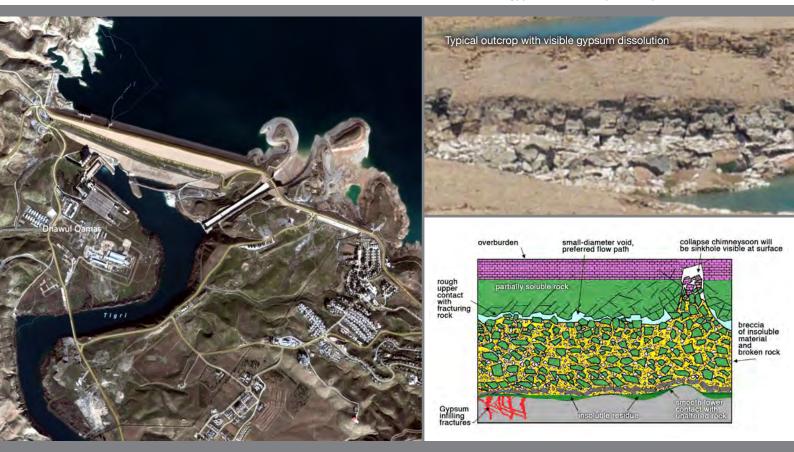
Introduction

Mosul Dam is an earth fill dam, located in Northern Iraq, on the Tigris River, approximately 40 km northwest of Mosul. The construction of the dam was completed in 1984. It is one of the largest multi-purpose dams in the Middle-East, and the largest Dam in Iraq.

The dam is a multi-purpose dam providing flood control, irrigation, power generation, and water supply. Through its 113 m height and 3.65 km length, the reservoir has a storage capacity of 11.1 billion m³ of water. The main hydroelectric power station has 4 turbines for an installed capacity of 750MW. The discharge system foresees two bottom-outlet tunnels (10 m in diameter), a five-gate service spillway, and an emergency fuse-plug spillway. the foundation rock, along the longitudinal axis of the Dam, before placing the embankment; the gallery was specifically designed also to allow repeated grouting efforts to continue over the lifetime of the dam.

The original specialized subcontractor initiated an aggressive grouting program in 1984, but the impoundment of the reservoir began whilst the curtain grouting was in progress. Afterward, the Iraqi Ministry of Water Resources (MoWR) has continued grouting ever since 1989, using the equipment and technique left at site by the subcontractor.

In spite of the actions taken, over the years the structural integrity of the dam and its operational capacity were cause for concerns, mainly because of the evident phenomena of dissolution of the gypsum and anhydrite layers. The rate of



A problematic geological setting

The dam is founded on a layered sequence of rocks including marls, chalky limestone, gypsum, anhydrite, and limestone. A feature of the geology is the occurrence of karstic limestone and the development of solution cavities within the gypsum and anhydrite layers. Dissolution phenomena result in the formation of fissures and voids that allow considerable water percolation, due to the strong hydraulic gradient imposed by the Dam. The karst development extends to a depth of 100 m below the dam's foundation.

The under-seepage phenomena and the associated risks were identified during the design for the dam construction, and actions were taken in an attempt to mitigate the possible outcomes. To this aim, a deep grout curtain was designed to be installed all along the dam centerline, from a grouting gallery. The grouting gallery was constructed on top of subsurface dissolution was increased by the presence of the reservoir; above a pool elevation of 318 m, the rate of subsurface dissolution increased markedly, leading to the recommendation that the pool not be raised above this level.

The low reservoir level impacted the power generation and the agricultural irrigation. In addition, the geopolitical instability in Northern Iraq almost interrupted the ordinary operations of maintenance grouting (managed by MOWR) between 2014 and 2015; and it is believed to have further impacted the condition of the Dam foundation.

The tender for the remedial works

In **2016, the U.S. Army Corps of Engineers** (USACE) performed a risk-assessment analysis and identified various Potential Failure Modes at the highest risk level, all related to the risk of internal erosion through the foundation bedrock. The risks for Mosul Dam were classified as very high, mainly due to the dam location and the number people living in along the Tigris Valley: if the risks were not addressed, failure can result in catastrophic loss of life, economic damage, and geopolitical instability.

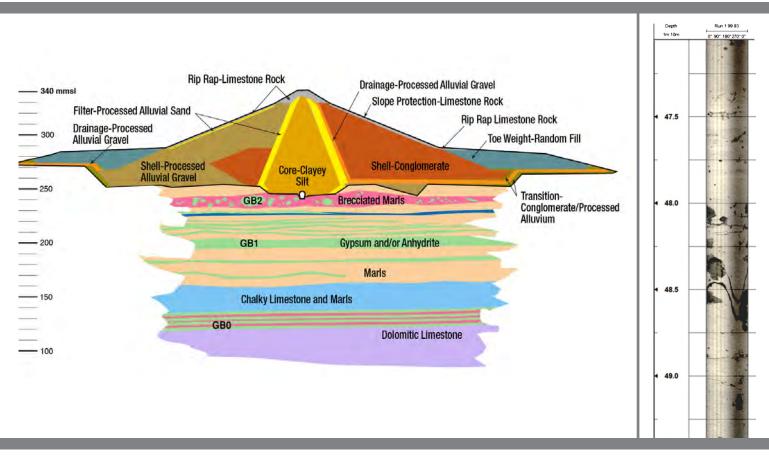
Owing to the critical situation, the Government of Iraq awarded the Mosul Dam Rehabilitation Project to the Italian firm Trevi S.p.A. (Trevi Group), through an international bid launched in October 2015. The Contract between the Iraqi Ministry of Water Resource (MoWR) and TREVI was signed in early March 2016. The Government assigned the roles of Engineer and Contract Administrator to the USACE, monitoring Mosul Dam since May 2015.

The contract included all the activities aimed at repairing the

buildings, of a new repair-maintenance shop, of the grout and mortar mixing plants. Concurrently, activities were started for the replacement of the entire grouting infrastructure and the construction of new networks for electric-power, ventilation, communication and internet, water and wastewater systems.

Drilling and grouting works

The drilling and grouting activity for the Dam refurbishment started at the end of October (2016). The drilling and grouting works were carried out with the goal of injecting mixes able to intercept and plug the fissures and voids (mainly derived from dissolution and karst phenomena). The end purpose of the emergency grouting program was to install a grout curtain along the full length of the grouting gallery (2200 m) and



grout curtain, with the supply of all the equipment and materials necessary to execute the drilling and grouting intervention. The contract included as well the works for the repair and maintenance of structures associated with the bottom outlet tunnels.

Camp and site preparation, and infrastructure upgrading

Mobilization and site preparation faced logistic problems related to the site location, with the war conflict against ISIS at 13 km from the Dam.

The setting up of the site began in July 2016, with the construction of a secure base camp facility to provide living and working accommodations for the approximately 1400 people on site. Also, it was started the installation of six new office at the East and West Abutments (490 and 220 m). A further piece of curtain was installed from the crest of the Main Dam in the 280 m immediately right of the spillway, to grout the rock between the base of the embankment and the top of the grouting tunnel extrados (where the grouting gallery enters the foundation rock). Owing to the stratigraphy and design considerations, it was necessary to drill and inject holes to a depth of 150 m and over.

As a result of the water level in the reservoir, the gallery activities were further complicated by the need to operate in the presence of a significant water-head (up to 100 m).

Drilling and Grouting Equipment

At the main dam, the injections were executed from the 2200 m long gallery, resting on top of the foundation rock, at the base of the impervious core. The gallery is accessible only

through two long and small adits located at left and right banks. The features of the gallery (3.7 m high and 3.0 m wide, with slopes up to 41%) posed significant challenges in terms of equipment configuration and maneuvering requirements, entailing the use of small-size rigs and grouting plants. **SOIL-MEC supported TREVI in customizing the machines with special features.**

For the production and delivery of the base grout mix, TRE-VI deployed three Main Mixing Plants (MMPs), installed outdoor and strategically located to feed the new grout pipelines placed along the crest and the grouting gallery. Each automated mixing plant included a **Soilmec SGM-45** (*capable to produce 30 m³/hr of grout*) equipped with twelve 45 m³ silos (for cement, water, dry and hydrated bentonite) and four and SM-16) were supplied for the outdoor drilling works. The three SM-16 (*long-mast rigs*) were equipped with a powerful double rotary, to allow the simultaneous driving of auger rods and steel casing for drilling the embankment.

All the rigs were equipped with a drilling parameter recording device (DPR for penetration and rotation speed, thrust and torque pressure, fluid pressure, specific energy).

Drilling and Grouting Method

According with the outcomes of the Risk Assessment Analysis (USACE, 2016), the Dam's foundation was divided into "Critical Areas", which defined the grouting priorities. At Mosul Dam, the grout curtain included three lines of holes. Each line comprised Primary, Secondary, Tertiary, Quaternary



horizontal piston pumps (**Soilmec SGP-20.12**). The pumps SGP-20.12 delivered the fresh grout mixture from the MMPs to the grouting stations, while keeping it in circulation through a 20-Km long closed circuit split in 6 pipelines.

Twenty grouting stations were located in the gallery and on the dam crest, nearby the grouting points. Each station included an automated batching grouting unit (*BGU*) type Soilmec SGP-9.12M, equipped with a vertical piston pump and a dosing-dilution system, to prepare the design mixes as needed.

Six Soilmec SM-5E electric crawler-mounted rigs were supplied for the drilling works inside the gallery. Few months later, four additional rigs were provided (*SM-5EM*). This new type of rig was custom designed to better fit within the limited space available in the gallery; also, allowed the drilling works under the spillway, where the gallery is characterized by a furtherly reduced section (2 m wide, 3 m high). Seven diesel rigs (*SM-5* holes, split spaced, from 12 m to 1.5 m. Quinary holes were drilled in between, mainly for coring and verification purposes. Some additional boreholes were instructed, on the basis of the grouting and verification core results. Locally in the gallery, the grout curtain was integrated with an extra row of boreholes downstream inclined.

Almost all the boreholes were grouted to a depth ranging between 100 and 150 m depth. The holes bored from the crest of the dam involved the drilling of 10 to 40 m of embankment and core. The drilling through the embankment was performed by auger and temporary casing *(without any drilling fluid)* to prevent any possible hydraulic/pneumatic fracturing of embankment and clay core. A permanent plastic casing was cemented inside the borehole *(in the embankment and embedded 1 m into the rock beneath)* for the upcoming drilling and grouting of the foundation rock. The drilling through the foundation rock was always performed by rotary system, using water as the flushing medium. Most of the drilled boreholes were vertical or subvertical. However, several boreholes were to be drilled in inclined array, to allow the continuity of the curtain under the seven water supply tunnels in the foundation beneath the grouting gallery (the tunnels are related with the outlet structures, the power plant and a nearby pump storage facility). Further inclined boreholes were drilled and grouted to connect the grout curtain installed from the East bank (surface) to the curtain installed under the concrete structure of the spillway.

The upstage method ($\leq 5 \text{ m long stages}$), was the most applied grouting method. The downstage and downstage-zone methods were also used, where needed by the borehole in-

and observed issues while drilling. The maximum effective pressure to be used in each borehole at the different depths was determined on the basis of specific considerations, to get the best penetration while limiting the risk of rock hydro-fracturing. The completion criteria for each stage were based on the designed refusal pressure and flowrate, which had to be reached and maintained constant for a prefixed period of time. Once the closure condition was reached, a further waiting time was spent to allow the complete dissipation of the final back-pressure.

All the grouting operations were controlled, monitored and managed by "T-Grout", a Trevi proprietary Automated Control System of the Grouting Process. The grouting software directly governs the whole grouting process and triggers mix



stability. For all cases, each borehole was pressure-grouted using a single packer, lowered in the hole at top of the stage to be grouted.

A series of stable grout mixes, characterized by different cement content, viscosity and setting time, were designed and tested. The aim was to have grouts able to treat both the large voids and the extensive net of fine fissures that characterize the foundation rock.

As a rule, the grouting of each stage was started with a thin grout (both in density and viscosity), and continued with thicker grouts, with increasing cement content and setting fastener. The grouting volume for each type of mix was directed by a flowchart defining the relationship between the effective pressure recorded during grouting and the volume of mix being injected. The grouting pressure and methodology varied along the dam, based on encountered geology changes based on pre-set criteria for pressures or volumes within the bounds of the refusal criteria. All the collected data are transmitted to the grouting software, which in turn regulates the pump pressure and flow-rate to maintain their values within the required limits. The software automatically computes the effective pressure, correcting the "raw value" measured at the borehole collar, taking into consideration all the dynamic and static head losses/gains. Wireless hotspots with internet connection were setup across the entire dam to enable communication among teams working simultaneously throughout the dam and the offices.

To this aim, the entire gallery and crest were equipped with a Local Area Network (LAN) using 2500 m of fiber-optic cable newly installed. With the help of the T-Grout system, more than 900,000,000 data were recorded and stored. This huge amount of data made possible the deep analyses of the

403,115 m of grouted boreholes

man-hours worked without accidents

8,000,000



1000 units workforce

5,393 grouted boreholes grouting behavior and foundation rock responses.

The drilling and grouting works included all the complementary activities, such as water pressure tests, coring, physical and electronic survey, etc.

Electronic equipment was extensively used to investigate the conditions of the rock, before and/or after grouting. The borehole optical televiewer (OPTV), capable of creating a high-resolution photographic image of the full circumference of the borehole, is a very useful instrument to provide reliable information on the state of the rock (and borehole deviation) in lieu of the time-consuming traditional coring. The use of the high-resolution acoustic televiewer (HRAT) allowed the surveying of boreholes in the presence of turbid water. The closed-circuit television camera (CCTV) was used to genernificant efforts to complete the grout curtain and to install the upstream piezometers. As expected, the largest grout takes were recorded in the vuggy limestone and GB layers of the Lower Gallery.

In general, the downstream line (the first injected) recorded the highest grout takes. Within each line, the grout takes significantly decreased from the primary holes towards the secondary, tertiary and quaternary holes. The decreasing trend in terms of grout take is a clear signal of the void filling and gradual reducing of the residual permeability. In addition to the holes inspected by OPTV, check holes were cored-drilled and investigated by water pressure tests to evaluate the grouting efficacy. The 81% of the tested stages measured values LU \leq 1 and the 98% LU \leq 3. In general, the result of



ate electronic high-resolution video of boreholes above the groundwater level. The high-resolution impeller flowmeter (HRIF) was used to measure the artesian flow rate encountered in the boreholes at various depths.

Grouted quantities and results

The drilling and grouting works were performed 24 hours/ day, six days/week. From November, 2016 through July, 2019, Trevi completed 403,100 m of grouted holes injecting over 40,800 m³ of grout mixes, with an average grout take of 102 l/m. The activity involved 667,800 m of rotary drilling/ redrilling, 85,000 m of coring, 35,450 m predrilling in the embankment.

The boreholes in the lower part of the gallery were drilled and grouted in challenging conditions, due to very high reservoir level. Here, the artesian pressure and water flow required sigthe Lugeon tests was evaluated as a great improvement. Besides the drilling and grouting works, activities were performed to install and renew the inclinometer and piezometer array, both downstream of the dam and into the gallery.

These instruments allowed the monitoring of the hydraulic pressures in the dam foundation bedrock, and their behavior as the grouting activity progressed. The readings showed an evident amelioration of the hydraulic conditions, with noticeably increasing of the hydraulic gradient between upstream and downstream piezometers.

A new risk analysis downgraded the risk to a level significantly lower than the one evaluated in 2016. The general safe conditions of the Dam allowed for the water to rise again at levels of 2005 and for the reopening of the spillway.

Rehabilitation of the bottom outlet tunnels

An important part of the contract was related to the rehabilitation of the Bottom Outlets. The first operation performed was the repair of the Guard Gate of the West Bottom Outlet, that was broken in closed position since 2013. Trevi managed to safely repair the indicator rod that was preventing the opening of the guard gate, restoring the full functionality of the guard gate system. The tunnel was reopened in October 2016, and water flowed through the west bottom outlet for the first time since 2013. The guard gates were inspected and found to be in good condition. New indicator rods were designed and installed at both the gates. tunnels, are structures with 10 m diameter in the steel part and 12 m in the concrete section: all the joints and expansion joints were inspected and repaired where necessary.

A structure with dentates was designed and installed at the bottom outlets to dissipate the water energy during operation. Bathymetric surveys and hydraulic computer models were applied to evaluate the scour potential in the plunge pool. The outcomes were used to appraise methods for the plunge pool stabilization.

Training of local staff

Under the Contract, Trevi had to train the local MoWR staff and technicians for the operation, maintenance and repair of the drilling and grouting equipment. It was an important



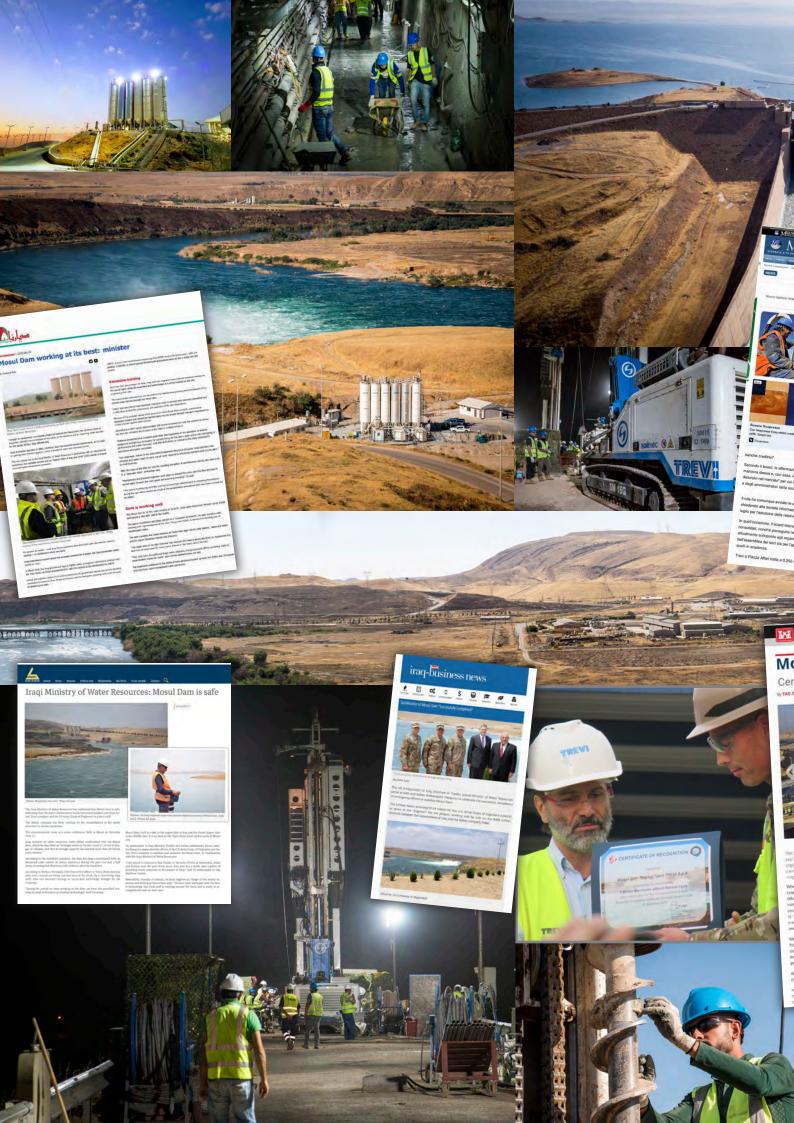
Trevi managed also the maintenance and repair of all the dewatering system of the tunnels, and of the four main gantry cranes located at the guard gate tower, at the bottom outlet regulating structure, at the spillway, and at the hydropower intakes.

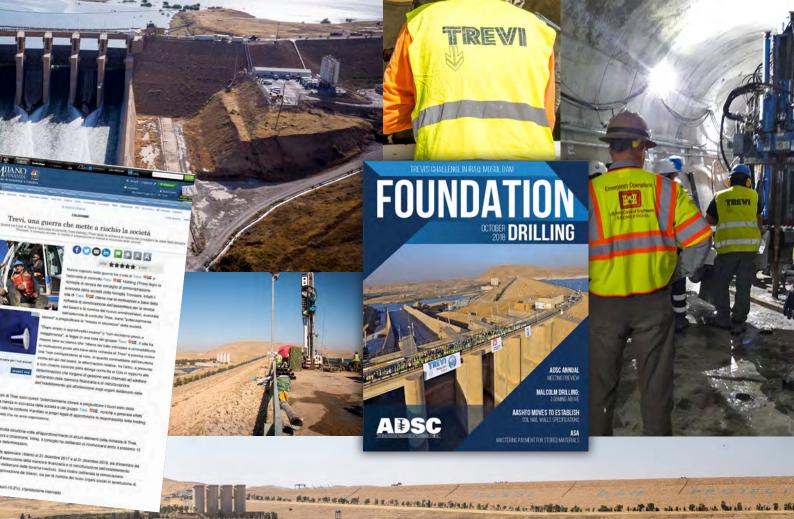
Another remarkable feature was the inspection and maintenance of the steel bulkheads. The bulkheads are lodged in a concrete structure located into the lake at about 40 m depth. With the deployment of a dedicated pontoon and a specialized crew including 6 divers, the bulkheads were lifted to the surface and fully inspected. Inspection and refurbishment were successful, and bulkheads were returned to their storage position in the reservoir.

Having the bulkhead in place, it was possible the dewatering of the upstream culverts and the inspection of the whole length of the twin bottom outlet tunnels. These 700 m-long issue, because the drilling and grouting works will have to be carried out through the entire life of the dam.

To engage the Owner's personnel to a learning process for the use and application of the modern equipment and techniques, TREVI held specific and exhaustive training courses for a total of 155 among engineers, technicians and operators. Theoretical classes and field activities were organized for the Iraqi personnel to familiarize with the new equipment, methodologies and technologies.

The USACE-Trevi team had further extended the training concept to include the integration of MoWR personnel into the Contractor's crews, with the aim of coaching the MoWR work force to directly execute the work on their own in the upcoming years, with the newly implemented techniques, equipment and software. Integration was achieved by establishing effective approaches for knowledge transfer to the





osul Dam Task Force celebrates mission completion

emony marks return of Grouting and Maintenance Operations back to Iraqis



Sector I

une 16, 2019







Iraqi staff, overcoming the language differences and bridging technical barriers.

Conclusion

In the years, the workforce reached a pick of about 700 units, with Italian management and supervision, third-country national staff and local personnel.

Trevi commenced the works with the war conflict against ISIS at 13 km from the Dam. The presence of the Coalition Forces, together with the Italian Army, guaranteed the required security of the Project Area. Notwithstanding the strict security procedures, the works proceeded expeditiously with no delays till the end of the Contract (*July 2019*).

About 8-million man-hours were worked without recordable incidents.





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